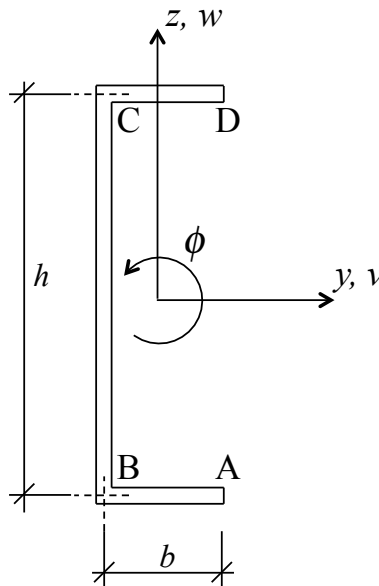


Channel cross-section

The open cross-section shown in the figure below is considered. The objective is to calculate all cross-section constants and determine all stresses due to given stress resultants. The thickness of all parts of the cross-section is denoted t . The cross-section is “thin-walled” and notice that the dimensions are given to the centre-lines of the cross-section parts.



Input values

Dimensions:

$$b = 80 \text{ mm ;}$$

$$h = 250 \text{ mm ;}$$

$$t = 10 \text{ mm ;}$$

Stress resultants:

$$N = 50 \text{ kN} ;$$

$$M_y = 50 \text{ m kN} ;$$

$$M_z = 5 \text{ m kN} ;$$

$$V_z = 5 \text{ kN} ;$$

$$V_y = 5 \text{ kN} ;$$

$$T \text{StV} = 0.5 \text{ m kN} ;$$

$$B = 0.5 \text{ kN} * \text{m}^2 ;$$

Axial force

The relevant cross-section constant is in this case the cross-sectional area:

$$A = 2 b t + h t$$

which yields: 4100 mm^2

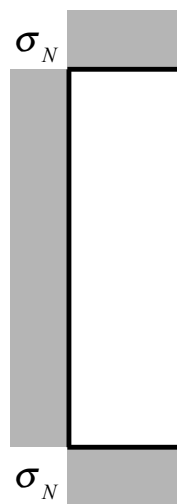
The axial stress is the axial force divided by the cross-sectional area:

$$\sigma_N = \frac{N}{A} ;$$

$$\text{UnitConvert}[\sigma_N, "N/\text{mm}^2"] // N$$

which yields: $12.1951 \text{ N}/\text{mm}^2$

The axial stress is distributed uniformly over the cross-section as shown here:



Bending about the y-axis

The relevant cross-section constant is in this case the moment of inertia of the cross-section about the y-axis. Notice that for thin-walled cross-sections it is convenient and sufficiently accurate to neglect

the local moment of inertia of cross-section parts that are aligned parallel with the y-axis. For those parts we include only the term dictated by the parallel axis theorem (Steiner's sats), namely area times distance to the neutral axis squared:

$$I_y = \frac{t h^3}{12} + 2 b t \left(\frac{h}{2} \right)^2 // N$$

which yields: $3.80208 \times 10^7 \text{ mm}^4$

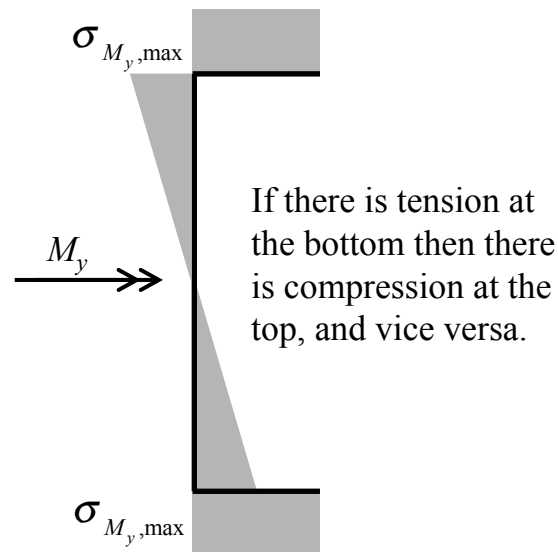
The maximum stress appears at the outermost fibre:

$$\sigma_{MyMax} = \frac{M_y h}{I_y 2};$$

$$\text{UnitConvert}[\sigma_{MyMax}, "N/mm^2"]$$

which yields: 164.384 N/mm^2

This stress is distributed over the cross-section as shown here:



Bending about the z-axis

Here we must first determine the location of the neutral axis along the y-axis, relative to the centre-line of the web, denoted y_0 and shown in Figure 4. This distance is the “first moment of area” about that reference point, divided by the total area:

$$y_0 = \frac{(2 b t) \frac{b}{2}}{A} // N$$

which yields: 15.6098 mm

The moment of inertia is as follows, again neglecting the local moment of inertia for the cross-section part that is parallel to the z-axis:

$$I_z = \left(\frac{h t^3}{12} + h t y_0^2 \right) + 2 \left(\frac{t b^3}{12} + b t \left(\frac{b}{2} - y_0 \right)^2 \right)$$

which yields: $2.43514 \times 10^6 \text{ mm}^4$

The axial stress in the web, i.e., at any location from B to C is:

$$\sigma_{MzB} = \frac{Mz}{I_z} y_0;$$

`UnitConvert [σMzB, "N/mm2 "]`

which yields: 32.051 N/mm²

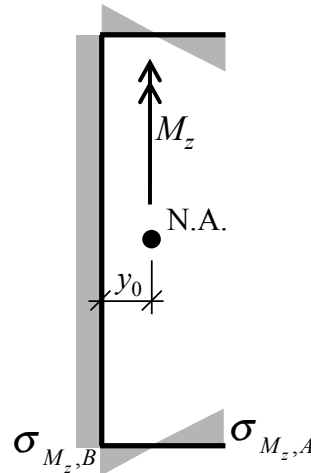
The axial stress at the tip of each flange is

$$\sigma_{MzA} = \frac{Mz}{I_z} (b - y_0);$$

`UnitConvert [σMzA, "N/mm2 "]`

which yields: 132.21 N/mm²

This axial stress is distributed over the cross-section as shown here:



Shear stress due to shear force in the z-direction

The shear stress is computed by the formula

$$\tau = \frac{V}{I} Q$$

where Q is the first moment of area about the point where the stress is calculated. Using that formula, the shear stress at B is:

$$\tau_{VzB} = \frac{Vz}{I_y t} \left(b t \frac{h}{2} \right);$$

UnitConvert [τ_{VzB} , "N/mm2"]

which yields: 1.31507 N/mm²

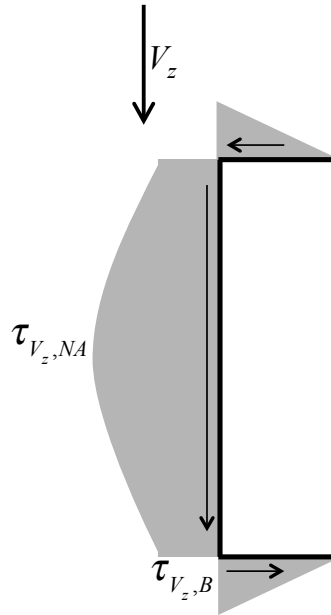
At the neutral axis the shear stress is:

$$\tau_{VzNA} = \tau_{VzB} + \frac{Vz}{I_y t} \left(t \frac{h}{2} \frac{h}{4} \right);$$

UnitConvert [τ_{VzNA} , "N/mm2"]

which yields: 2.34247 N/mm²

The shear stress is distributed as follows:



Shear centre by equilibrium

Now that we have the shear stress due to a shear force in the z-direction it is possible to determine the shear centre by equilibrium considerations. For that purpose, we first calculate the force resultant from the shear stress in each cross-section part. In the “flanges” that resultant it:

$$V_z \text{Flange} = \frac{1}{2} \tau V_z B t b$$

which yields: 0.526027 kN

In the “web” it is:

$$V_z \text{Web} = \tau V_z B t h + \frac{2}{3} (\tau V_z NA - \tau V_z B) t h$$

which yields: 5. kN

Then the location of the shear centre, relative to the centre-line of the web, is determined by equilibrium about the shear centre, which is located an unknown distance from the centre of the web:

```
equation = VzFlange h == VzWeb distance;
solution = Solve[equation, distance];
ySC = (distance /. solution) [[1]];
UnitConvert[ySC, "mm"]
```

which yields: 26.3014 mm

Shear stress due to V_y

Using the same approach as above, the shear stress at the neutral axis is

$$\tau_{VyNA} = \frac{V_y}{I_z t} \left(t \frac{(b - y_0)^2}{2} \right);$$

$$\text{UnitConvert}[\tau_{VyNA}, "N/mm^2"]$$

which yields: 4.25653 N/mm²

Stress at corners:

$$\tau_{VyB} = \tau_{VyNA} - \frac{V_y}{I_z t} \left(t \frac{y_0^2}{2} \right);$$

$$\text{UnitConvert}[\tau_{VyB}, "N/mm^2"]$$

which yields: 4.00638 N/mm²

Here we also check that the shear stress at the midpoint of the web is zero:

$$\tau_{VyMidweb} = \tau_{VyCorner} - \frac{V_y}{I_z t} \left(t \frac{h}{2} y_0 \right)$$

which yields: $\tau_{VyCorner} + -0.00400638 \text{ kN/mm}^2$

The following figure shows the distribution of the shear stress over the cross-section:

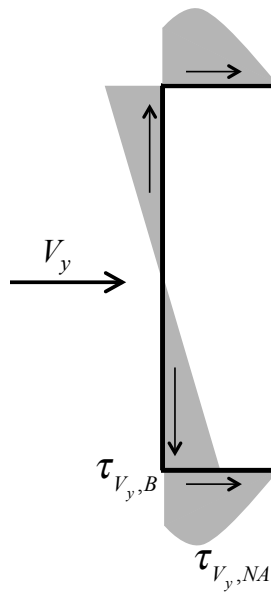


Figure 6: Shear stress due to shear force in the y-direction.**Shear stress due to $T_{st.v}$.**

The cross-section constant for Saint Venant torsion is

$$J = \frac{1}{3} h t^3 + 2 \times \frac{1}{3} b t^3 \quad // \quad \text{N}$$

which yields: $136\,667. \text{ mm}^4$

The shear stress is obtained from Prandtl's stress function, which in this case reads

$$\Phi = k \left(1 - 4 \frac{r^2}{t^2} \right);$$

Differentiation with respect to r yields the shear stress:

$$\tau_{StV} = \left(\frac{8 k}{t^2} \right) r;$$

The maximum shear stress appears at the edge:

$$r = \frac{t}{2};$$

The constant k is determined from the stress resultant equation $T = \text{two times the volume under the stress function}$:

$$\text{equation} = T_{StV} = 2 \times \frac{2}{3} k t (2 b + h);$$

Solving yields the following k -value:

```
solution = Solve[equation, k];
k = (k /. solution) [[1]];
UnitConvert[k, "N/mm"]
```

which yields: 91.4634 N/mm

In turn, this yields the following maximum shear stress:


```

τStV /. solution;
UnitConvert [ τStV, "N/mm2 " ]

```

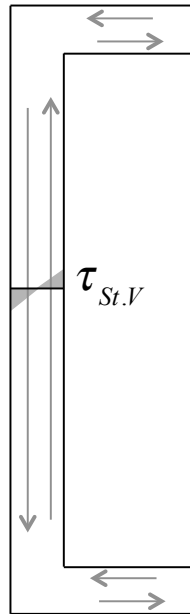
which yields: 36.5854 N/mm²

Those calculations can be combined into the formula given in the notes, which gives the same shear stress value:

$$\text{UnitConvert} \left[\left(\frac{8 \frac{t}{2}}{t^2} \right) \left(\frac{3 TStV}{4 t (2 b + h)} \right), "N/mm2 " \right]$$

which yields: 36.5854 N/mm²

The distribution of the shear stress over the cross-section is shown here:



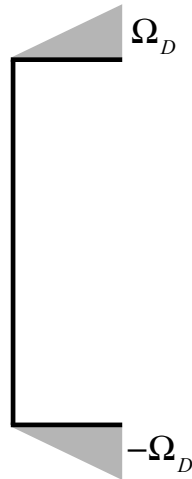
Axial stress due to bi-moment B

A trial omega diagram is first established about the trial point Q, selected at mid-height of the web. The value at D is:

$$\Omega_{QD} = b \frac{h}{2}$$

which yields: 10 000 mm²

The trial omega-diagram is shown here:



The integral of this trial diagram is zero, hence the normalization constant C is zero. However, the integral of $z \Omega$ is nonzero:

$$\text{integral} = \left(\frac{h}{2} \right) \left(\frac{1}{2} t b \Omega_{QD} \right) + \left(-\frac{h}{2} \right) \left(-\frac{1}{2} t b \Omega_{QD} \right)$$

which yields: $1\,000\,000\,000 \text{ mm}^5$

That gives the same location of the shear centre coordinate relative to the centre of the web as calculated earlier:

$$y_{scFromWeb} = -\frac{\text{integral}}{I_y}$$

which yields: -26.3014 mm

In turn, that gives the following shear centre coordinate relative to the neutral axis:

$$y_{sc} = -y_0 + y_{scFromWeb}$$

which yields: -41.9111 mm

z_Q and z_{sc} is zero due to symmetry. The final omega diagram is $\Omega = \Omega_Q + (y_{sc} - y_Q) z$ where we notice that

$$y_Q = -y_0;$$

The final omega diagram has the following key values:

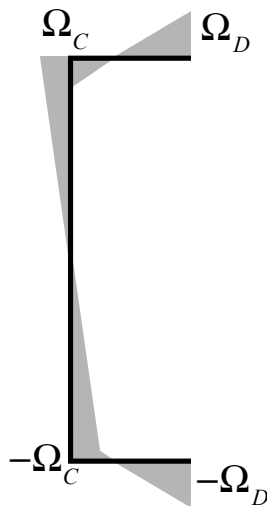
$$\Omega_D = \Omega_{QD} + (y_{sc} - y_Q) \frac{h}{2}$$

which yields: 6712.33 mm^2

$$\Omega_C = (y_{sc} - y_Q) \frac{h}{2}$$

which yields: -3287.67 mm^2

That omega diagram looks like this:



To obtain the cross-sectional constant for warping torsion we use “quick integration formulas” to integrate the omega diagram:

$$C_w = 2 t \left(\frac{1}{3} \Omega_C^2 \frac{h}{2} + \frac{1}{3} \Omega_C^2 b + \frac{1}{3} \Omega_D^2 b - \frac{2}{6} \text{Abs} [\Omega_C] \text{Abs} [\Omega_D] b \right)$$

which yields: $2.7032 \times 10^{10} \text{ mm}^6$

The axial stress is proportional to the omega diagram shown above:

$$\sigma_D = \frac{B}{C_w} \Omega_D;$$

UnitConvert [σ_D, "N/mm²"]

which yields: 124.155 N/mm^2

$$\sigma_C = \frac{B}{C_w} \Omega_C;$$

`UnitConvert` [σ_C , "N/mm²"]

which yields: -60.8108 N/mm^2